

ANALYSIS ON VARIOUS TYPES OF AGGREGATES ON PROPERTIES OF CONCRETE

Gana. A. J.; Ineye. D & Braimoh S.O.

Department of Civil Engineering Landmark University Omu- Aran, Kwara state Email: pholdebo123@gmail.com

ABSTRACT

This study revealed the substitute for conventional materials used in the production of concrete. It involves the use of various aggregates like laterite and quarry dust to replace conventional river sand fine aggregate. The full replacement of granite as coarse aggregate in concrete with less utilized local materials like bush gravel and limestone was employed. Samples of concrete cubes were made using replacement cements of laterite and quarry dust and samples of cubes made by using limestone and gravel instead of granite. The quality of laterite and quarry dust as replacement varied from 0%, 25%, 50%, and 75%, 0% being the control; while granite was fully replaced to ascertain qualities of concrete using a whole new material. A total of 68 cubes of standard 150 mm x 150mm x 150mm were cast from the various batches and cured in a water tank at ambient temperature. The samples were cured for specified periods of 3 days, 7 days, 14 days and 28 days Workability test (slump test) was carried out to determine optimum water content noted at 0.5 water/cement ratio for strength. It was observed that the weight of the specimen decreased as the percentage of replacement for both laterite and quarry dust increased. Bulk density of concrete reduced as replacement percentage for sand increased; for coarse aggregates gravel gave the highest value of 2479kg/m³. Compressive strength test and flexural test were carried out for the samples (cubes and beams respectively), and it was observed that compressive strength for all sample batches increased as curing age increased. Furthermore, observations showed for both laterite and quarry dust replacement that compressive strength increased as percentage replacement increased up to 25%. Thereafter, it decreased. Limestone gave a better strength overall than granite and gravel as full replacement of coarse aggregate with a value of 14.47 N/mm^2 .

Keywords: Comparative Analysis Effects various Aggregates properties concrete

INTRODUCTION

In recent times, studies have been carried out to determine the usefulness of laterite, excess quarry dust mined from aggregates and other relatively abundant mineral materials in various countries of the world, and other abundant man-made by products similar in physical properties to such abundant element such as construction waste like crushed sand-Crete blocks and other materials of abundance in the country. Furthermore,

Nigeria is a country in the tropics where its climate ranges from semiarid in the north to humid in the south (Adewumi, 2019). Soils such as laterite are abundant in Nigeria, and are being utilized heavily in Nigerian construction industry (Yaragal et al., 2019). Laterite has its use in almost all aspect of civil, Laterite was used extensively in the construction of embankments for roads and earth dams as indicated by (Makasa, 2004). Also, with the high level of waste generated, countries have put up policies to reduce waste generated with recycling and reuse of these waste materials. A lot of waste is generated in construction process or in the remodeling or demolition of structures. In this sense, reuse of construction waste is paramount for both the environment and also as an aid to reduce cost of constituents used for concrete production. The aim of this paper is to create awareness of the usefulness of abundant materials and even materials termed 'wastes' and impact of aggregates on the properties of concrete, the use of other un-popular coarse aggregate and even the mix ratios of these mineral combinations and to compare the values gotten from using various aggregates to a control, re-evaluate from previous publications the optimum percentage of these supplementary materials, re-analyze their effects on concrete and finally compare these fine aggregate materials using other standard coarse aggregates usually employed in Nigeria.

MATERIALS AND METHODS

The materials used in this in this study are water, fine aggregate and quarry dust. The samples of concrete cubes were made by weighing the materials with standard. Equipment in the laboratory, and the curing of those cubes inside water tanks was carried out for all the cubes. A specific number of Tests were carried out, with results were later analysed.

Test on Fine Aggregates

Sieve Analysis and Physical Properties

The study, as one of the main objectives, was to determine the physical and mechanical properties of all fine and coarse aggregates to be used in course of experimentation. The results are shown below in the tables. The table shows the sieve analysis of the fine aggregates used, which included the conventional sand and laterite, and also the result as well for the



coarse materials. The results from the graph plotted revealed the fines are well graded



The figure above showed the grading curve obtained for the fine aggregates. The result of particle size of fine aggregates used include: sand which ranged from 0.055 to 2mm with a specific gravity of 2.62 highlighted in the table- in appendix. Uniformity coefficient of sand was found to be 3.06 and coefficient of curvature 1.02, and sand is can be said to be well graded with particles passing the 4.75m sieve and retained on 75μ sieve. Quarry dust gave a fineness modulus of 2.92 with the sieve analysis and other properties tabulated below.

rable 4.1. Specific gravity and water absorption						
`	LIMESTONE	GRANITE/CA	FA	LATERITE	STONE DUST	GRAVEL
Mass Of Bottle + sample + Water (M_3)	524.5	534	90	83.5	78	1097
Mass Of Bottle + sample (M2)	275	271.5	46.5	36.5	30.5	453.5
Mass Of Bottle Full Of Water Only (M4)	421	427	73	`	73	767.5
$\mathcal{M}ass \ Of \ Bottle \ (\mathcal{M}I)$	103	105.5	19	19.5	20	35.5
Mass Of Water Used (M3-M2)	249.5	262.5	43.5	47	47.5	643.5
Mass Of sample Used (M2-M1)	172	166	27.5	17	10.5	418
Volume Of sample $(\mathcal{M}_4-\mathcal{M}_1)-(\mathcal{M}_3-\mathcal{M}_2)$	68.5	59	10.5	12.5	5.5	88.5
$G_{s} = (\mathcal{M}_{2} - \mathcal{M}_{I}) / (\mathcal{M}_{4} - \mathcal{M}_{I}) - (\mathcal{M}_{3} - \mathcal{M}_{2})$	2.51	2.81	2.62	1.36	1.91	4.72

Table 4.1: Specific gravity and water absorption

4.2 Test on coarse aggregate Table 4.2: Sieve analysis

Sieve Number	Diameter (mm)	Mass of Sieve	Mass Sieve	of &	Coarse Soil	Coarse Soil	Coarse Sc Passing (%)	oil
,	, y		Coarse	Soil	Retained	Retained		
			(\mathbf{g})		(\mathbf{g})	(%)		
	25.000	574	574		0.0	0.00	100.00	
	20.000	570	580.5		10.5	1.05	98.95	
	13.200	561.5	998.5		437.0	43.70	56.30	
	5.000	521.5	1074		552.5	55.25	1.05	
	2.000	525.5	525.5		0.0	0.00	0.00	
	1.180	494.0	494.0		0.0	0.00	0.00	
	0.600	477.0	477.0		0.0	0.00	0.00	
	0.425	454.0	454.0		0.0	0.00	0.00	
	0.300	449.0	449.0		0.0	0.00	0.00	
	0.212	420.0	420.0		0.0	0.00	0.00	
	0.150	402.0	402.0		0.0	0.00	0.00	
	0.075	367.0	367.0		0.0	0.00	0.00	
	0.063	381.5	381.5		0.0	0.00	0.00	
Pan		389.0	389.0		0.0	0.00	0.00	
					1000.0	100.00		



Figure 4.4: sieve analysis for coarse aggregate

The table 4.4 below, shows the values of the various test carried out on the coarse aggregate. It also specifies its compliance to predetermined requirement according to the standard, thereby making it suitable for use in various construction works.

Aggregate Impact Value

The result of the aggregate impact value test for gravel and granite is 23.9 and 19.8 respectively. This means impact value fall within desirable region. BS 822 [11] prescribed a maximum value of 45% for non-wearing surfaces. This value is inversely related to the toughness of aggregate, meaning a higher value connotes a lower toughness.

Aggregate Crushing Value

The test result for gravel and granite used are 38.8 and 29.8 respectively, which lies within expected value of 45% for ordinary cement used. For further emphasis there is indirect relationship between this value gotten and compressive strength of concrete.

Test carried out	Obtained Test Results (GRANITE)	Obtained Test Results (GRAVEL)	5 Standard Test Values
Aggregate Impact Test	19.8%	23.9	30% maximum
Aggregate Crushing Test	29.8%	38.8	45% maximum
Los Angeles Abrasion Test	33.8	34.8	60% maximum
Flakiness Index	24.8%	23.0	30% maximum
Elongation Index	25.2%	34.9	30% maximum
Density	1500.20kg/m^3	1650 kg/m³)	
Specific Gravity	2.81	2.98	3 Maximum

Table 4.3: Test carried out on Coarse Aggregate

Test on Limestone

Below are the sieve analysis and particle distribution for the limestone used in preparation of the unconventional concrete in the table. The values gotten are detailed in the table- found in the appendix.



Figure 4.5: sieve analysis for limestone



From the graph plotted, the values of D10, D30 and D60 on the fine aggregate curve was traced to be 4.5, 6.15 and 6.9 respectively. The Uniformity coefficient was found to be 1.52 and coefficient of curvature was 1.22 from the above figure. From ASTMD-2487 soil classification, it can be said that the Coarse aggregate is a well graded sample because $1 \leq C_{c \leq 3}$. The table shows the various results of test carried out on limestone.

Test carried out	Obtained Test Results	Standard Test Values
Aggregate Impact Test	32.7%	30% maximum
Aggregate Crushing Test	44.2%	45% maximum
Los Angeles Abrasion Test	47.2%	60% maximum
Flakiness Index	21.6	30% maximum
Elongation Index Density	20.7 1500.20kg/m ^ 3	30% maximum
Specific Gravity	2.51	3 Maximum

Table 4.4: Test carried out on Coarse Aggregate (Limestone)

The table also specifies its compliance to predetermined requirement according to the standard, thereby making it suitable for use in various construction works.

Test on Concrete

Test on fresh concrete (slymp test)

The result of the slump test are presented in the table below using a W/Cvalue of 0.6 to make concrete more workable. The figure 4.5 showed the gravel concrete produced the highest value of true slump due to its nonuniform gradation, presence of impurities i.e. silt

at	ole 4.5: slump va	lues for v	atiou	is replace	ment o	sand	
_	5. NO	% replaceme	of ent	slump (mm)	value	slump value(mm) (laterite)	Type of slump
_				(quarry u	ust/	(facefree)	
	I	0		10		10	l rue slump
	2	25		60		30	True slump
	3	50		20		40	True slump
	4	75		50		60	True slump

Table



Figure 4.6: slump value for 100% coarse aggregates utilized



Figure 4.7: slump values for replacement of sand

Effect of sand replacement inclusion on weight

It was observed that the weight of the specimen decreased as the percentage of replacement for both laterite and quarry dust increased. At the 3 days curing phase, weight gradually decreased with higher replacement values. At 0% replacement laterite value the weight was 7.86kg, at 25% the weight was found to be 7.5kg, at 50% the weight was found to be 7.03kg indicating a 6.2% reduction in weight, and at 75% it was 6.7kg; while for the quarry dust the 25% weight was found to be 7.5kg, 50% was 7.03 and 75% was 6.83kg

Bulk density of concrete



Table 4.6: Bulk Density of Concrete using Laterite as Replacement for Sand

5.NO	(kg)	VOLUME (m ³)	DENSITY (kg/m ³)	AVERAGE DENSITY ((kg/m ³)					
M30 REPLACEMENT									
I	7.4	0.003375	2192.59						
2	8.3	0.003375	2459.14	2330.82					
3	7.9	0.003375	2340.74						
25% REP	PLACEMENT LATERI	TE							
I	7.3	0.003375	2162.96						
2	6.9	0.003375	2044.44	2222.22					
3	8.3	0.003375	2459.25						
50% REI	PLACEMENT LATERI	TE							
I	6.7	0.003375	1985.19						
2	7.3	0.003375	2162.96	2083.95					
3	7.I	0.003375	2103.7						
75% REF	75% REPLACEMENT LATERITE								
Ι	6.2	0.003375	1837.04						
2	7.0	0.003375	2074.07	1985.18					
3	6.9	0.003375	2044.44						

Table 4.7: bulk density of concrete using quarry dust as replacement for sand

S.NO	WEIGHT OF CUBE	VOLUM	DENSITY(kg/m ³	AVERAGE					
	(KG)	E (m ³))	DENSITY					
				(kg/m ³)					
GRANITE									
Ι	7.4	0.003375	2192.59						
2	8.3	0.003375	2459.14	2330.82					
3	7.9	0.003375	2340.74						
25% REP	LACEMENT QUARR	y dust							
Ι	7.3	0.003375	2162.96						
2	6.9	0.003375	2044.44	2222.22					
3	8.3	0.003375	2459.25						
50% REP	LACEMENT QUARR	LY DUST							
Ι	6.7	0.003375	1985.19						
2	7.3	0.003375	2162.96	2083.95					
3	7.I	0.003375	2103.7						
75% REP	75% REPLACEMENT QUARRY DUST								
Ι	7.2	0.003375	2133.33						
2	6.9	0.003375	2044.44	2024.69					
3	6.4	0.003375	1896.3						



Figure 4.8: density of concrete using laterite as replacement





Figure 4.10: density of concrete using quarry dust as replacement for sand

A downward slope was noted for the bulk density graph, for both laterite and quarry dust indicating that the control (river sand) had a higher value in comparison to replacement samples. For coarse aggregate replacement, granite gave the highest value for bulk density owing to its particle size and gradation.



Water absorption

Table 1.8:	Water absor	ntion test res	ults after 2	8 davs	(laterite)
1 4010 4.0.	vv acci absor		and after 2	.o gayo	facorico

Batch	Block no	Dry mass	Wet mass	Water	Average water
		(kg)	(kg)	absorbed (%)	absorbed (%)
	I	8.370	8.4075	1.2	
control	2	8655.5	8.762	1.1	1.15
	Ι	7.662	7.967	3.98	
25 %	2	7.200	7.503	4.2	4.09
	Ι	5.5025	5.601	1.8	
50 %	2	6.201	6.311	I.77	I.79
	Ι	7.325	7.409	1.15	
75 %	2	6989.5	7.072	1.19	I.I 7

Table 4.9: Water absorption test results after 28 days (quarry dust)

Batch	Block no	Dry mass	Wet mass	Water	Average water
		(kg)	(kg)	absorbed (%)	absorbed (%)
	I	8.370	8.4075	I.2	
control	2	8655.5	8.762	I.I	1.15
	Ι	7.440	7.5315	I .2	
25 %	2	7.695	7.790	1.23	I.22
	Ι	7.464	7.539	I.O	
50 %	2	7.112	7.187	1.05	1.03
	Ι	6.801	6.866	0.95	
75 %	2	7.001	7.067	0.94	0.95

Table 4.10: Water absorption test results after 28 days

Batch	Block no	Dry mass (kg)	Wet mass (kg)	Water absorbed (%)	Average water absorbed (%)
	I	8.370	8.4075	I.2	
GRANITE	2	8655.5	8.762	I.I	1.15
	Ι	9.952	10.048	0.96	
GRAVEL	2	8.322	8.418	1.15	1.06
	Ι	7.205	7.275	0.97	
LIMESTONE	2	7.779	7.874	I.2	1.09

Compressive strength

The grade of concrete, type of aggregate used, age of curing are variables in this investigation. This test is done to determine the cube strength of concrete mix prepared. The test is conducted on 3days, 7days, 14days and 28days, with the details of the compressive strength of M15 gradesshown in the Table 4.8. The compressive strength values of percentage

replacement of sand 0%, 25%, 50% and 75% laterite and quarry dust are listed in details.

u.	. Compressive Ju	engun u	sing late	The as N	spracement	Joi Janu
-	% Replacement	mix	3 days	7days	14 days	28 days
_	Laterite	ratio				
			compressive		strength	
_			(N/mm	²)	-	
	0	1: 2: 4	5.14	8.45	11.70	13.01
	25	1: 2: 4	5.23	8.29	11.45	12.94
	50	1: 2: 4	4.74	7 . 7I	10.67	11.86
	75	1: 2: 4	4.4I	7.16	9.92	11.02

Table 4.11: Compressive Strength using Laterite as Replacement for Sand

Table 4.12: Compressive Strength using Quarry Dust as Replacement for Sand

% Replacement	mix	3 days	7days	14 days	28 days
Laterite	ratio				
		compres (N/mm	ssive 1 ²)	strength	
0	1: 2: 4	5.14	8.45	11.70	13.01
25	1: 2: 4	5.09	8.19	11.04	12.75
50	1: 2: 4	4.84	7.86	10.88	12.09
75	1: 2: 4	4.65	7.56	10.47	11.63

Table 4.13: compressive strength of concrete utilizing various coarse aggregate

Coarse	mix	3 days	7days	14 days	28 days
aggregate	ratio				
		compressive (N/mm²)		strength	
GRANITE	1: 2: 4	5.14	8.45	11.70	13.01
GRAVEL	1: 2: 4	4.57	7.38	10.04	11.19
limestone	1: 2: 4	5.69	9.36	13.02	14.47









Figure 4.12: compressive strength Using quarry dust as partial replacement of sand



Figure 4.13: compressive strength of concrete cube samples using different coarse aggregate and variation with curing age

The compressive strength of control sample surpassed all the replacement samples produced using laterite for the various replacement. It had a strength value of 6.84 N/mm^2 while the most suitable replacement using laterite produced 5.29 N/mm², which is a 29.3% difference. In all samples produced using laterite replacement, the control exhibited a more superior value and higher compressive strength, but observations show the 25% to 50% is an acceptable replacement value, for use in conventional construction work. The same could be said about the samples produced using quarry dust, the control is produced a superior strength for the experiments carried out. For the fine aggregate replacement, 75% of quarry dust proved to be the minimum compressive strength. There was an obvious increase in strength with increase in curing days for all samples.

Flexural Strength

The table 4.11 shows the details of the flexural strength of the different replacement for sand and types of coarse aggregate used for the concrete; at $_{3/7}$, 14 and 28 days for concrete grade M_{15} .



Table 4.14: Flexural Strength of Concrete using Laterite as Replacement for Sand

% Replacement	mix	3 days	7days	14 days	28 days
Laterite	ratio				
		Flexural		strength	
		(N/mm	²)	_	
0	1: 2: 4	1.79	2.29	2.7	2.85
25	1: 2: 4	1.82	2.27	2.67	2.84
50	1: 2: 4	1.72	2.19	2.58	2.72
75	1: 2: 4	1.66	2. II	2.49	2.62

Table 4.15: flexural Strength of Concrete using Quarry Dust as Replacement for Sand

% Replacement	mix	3 days	7days	14 days	28 days
Laterite	ratio				
		Flexural (N/mm²)		strength	
0	1: 2: 4	1.79	2.29	2.7	2.85
25	1: 2: 4	1.78	2.26	2.62	2.82
50	1: 2: 4	1.74	2.21	2.59	2.74
75	1: 2: 4	I.7	2.17	2.55	2.69

Table 4.16: flexural strength of concrete using various coarse aggregate

Coarse	mix	3 days	7days	14 days	28 days
aggregate	ratio				
		compressive (N/mm²)		strength	
GRANITE	1: 2: 4	1.79	2.29	2.7	2.85
GRAVEL	1: 2: 4	1.69	2.14	2.5	2.64
LIMESTONE	1: 2: 4	1.88	2.41	2.85	3.00



Figure 4.14: flexural strength using laterite as sand replacement

Figure 4.15: flexural strength using quarry dust as sand replacement

75%





CONCLUSION

Based on the systematic and detailed experimental study conducted on the various combinations used to produce concrete, with the aim of producing concrete of significant strength, M15 grade using locally available material, while still being economical in its approach; the following deductions can be drawn from the study:



- 1) The bulk density of concrete for all samples were around 2000 kg/m^3 , and the value of density decreased as replacement percentage increased.
- 2) Compressive strength of control, 25%, 50%, and 75% replacement of sand by laterite was found to be 13.01, 12.94, 11.86 and 11.02 respectively. While for quarry dust at 0%, 25%, 50%, and 75% replacement was noted to be 13.01, 12.75, 12.09, and 11.63 N/mm². For coarse aggregate granite, gravel and limestone gave compressive strength of 13.01, 11.19 and 14.47 after 28 days curing.
- 3) As the percentage of replacement increased, the workability of concrete increased especially for the Laterized concrete; the quarry dust replacement gave an irregular value as indicated by the slump test.
- 4) Flexural and compressive strength increased for all samples as curing age increased.
- 5) Limestone concrete gave the highest strength in comparison to other coarse aggregate used in concrete production.

In conclusion replacing sand with laterite and quarry dust are acceptable up to a replacement percentage of less than 50%, from whence a significant reduction of strength is noted from the investigations carried out, hence for construction work the suitable percentage of replacement especially structural is about 50%.

RECOMMENDATION

Although this study gives an insight on the effects of partial replacement of sand in concrete a lot of questions cannot be answered with the data at hand.

- It should be noted that partial replacement and not full replacement of sand as coarse aggregate should be welcomed and not feared in construction but done only when similar strength properties have been done on substitute like laterite and quarry dust.
- It is recommended that further investigations should be carried out on the effects of grading of the coarse aggregates used.
- Also, this investigation should be further expanded by combining the alternate coarse aggregate i.e. limestone and gravel with the various replacement materials for sand and note its characteristics.

- The use of granite and sand on conventional concrete cannot be overemphasized in its use in sophisticated construction like high rise buildings.
- Mix ratio was constant in this study, it is advised for further investigations that more mix ratios should be used to verify claims in this investigation.
- Further, a handful of experiments geared towards determining other structural properties of concrete should be added to further validate the claims touted in this study.

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